



PRODUCT APPROVAL SUPPORTING CALCULATIONS

1630 Horizontal Sliding Window – non-Impact

REPORT TO:

**MI WINDOWS AND DOORS, LLC
702 WEST MARKET ST
GRATZ, PENNSYLVANIA 17030**

REPORT NUMBER: 27488.05-107-16
REPORT DATE: 12/11/23

This item has been digitally signed and sealed by Michael D. Stremmel, PE on the date adjacent to the seal.

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Micheal D. Stremmel, PE
FL PE 65868
FL REG 37122

Scope

Molimo, LLC was contracted by MI Windows and Doors, LLC to evaluate alternate installation methods for their Series 1630 Horizontal Sliding Window. The evaluation is based on physical testing and product certifications. Reference standards utilized in this project include:

Florida Building Code, Building, 8th Edition (2023). International Code Council, 2023.

ANSI/AWC NDS-2018 National Design Specification (NDS) for Wood Construction. American Wood Council, 2018.

ADM1-2020 2020 Aluminum Design Manual. The Aluminum Association, Inc., 2020.

AISI S100-16 North American Specification for the Design of Cold-Formed Steel Structural Members, 2016. American Iron and Steel Institute, 2016.

ICC-ES Report ESR-1976 ITW Buildex TEKS Self-Drilling Fasteners. ICC Evaluation Service. 04/2021.

NOA 21-0201.06 Tapcon Concrete and Masonry Anchors with Advanced Threadform Technology. Miami-Dade County Product Control Section. 02/01/2021.

The anchorage analyses presented herein do not address the water resistance, water penetration or air infiltration performance of the installation method or the installed product. In addition, the analyses rely on the assumption that the building substrate is capable of withstanding incurred loads.

Certification of Independence

In accordance with Rule 61G20-3 Florida Administrative Code, Molimo, LLC hereby certifies the following:

- Molimo, LLC does not have, nor does it intend to acquire or will it acquire, a financial interest in any company manufacturing or distributing products tested or labeled by the agency.
- Molimo, LLC Laboratories is not owned, operated or controlled by any company manufacturing or distributing products it tests or labels.
- Micheal D. Stremmel, P.E. does not have nor will acquire, a financial interest in any company manufacturing or distributing products for which the reports are being issued.
- Micheal D. Stremmel, P.E does not have, nor will acquire, a financial interest in any other entity involved in the approval process of the product.

Analyses

Summary of Test Results

The following table summarizes the various 1630 Horizontal Sliding Window products and their corresponding performance levels which have been established by testing or product certification.

Table 1 Summary of Test Results

Series/Model	Test Report Number	Product Certification	Size (W x H)	Performance
1630 HS (XO) (Fin Install)	2343.02-106-12 (Rev. 1, 02/02/22)	CPD 18924	74" x 63"	+/- 50 psf
1630 HS (XO) (Finless Install)	2343.02-106-12 (Rev. 1, 02/02/22)	CPD 18924	74" x 63"	+/- 50 psf

Testing documented in Table 1 was conducted by Molimo, LLC of York, Pennsylvania (Florida Department of Business & Professional Regulation Test Lab No. TST11282, IAS Certificate of Accreditation TL-678).

As-Tested Installation Analysis

For air/water/structural testing, the test specimen was secured to a pine buck with #8 x 1-5/8" wood screws through the integral PVC nail fin. And, a test specimen with #8 x 2" screws installed through the window frame was also tested. The as-tested installation methods are evaluated on page 7 to page 14 and the established design capacities are summarized in Table 2.

Table 2 As-tested Anchorage Design Capacities

Test	Connection	Capacity
1630 HS Air/Water/Structural Test Nail Fin Install	#8 x 1-5/8" screws. Placed 3" from each corner and 8" on center.	48 lb
1630 HS Air/Water/Structural Test Finless Install	#8 x 2" screws. <u>Head</u> 4" from corners and 14" on center <u>Jams</u> 4" from corners and 14" on center <u>Sill</u> Steel installation clip at center of sill secured to window with two #10 x 1/2" screws and secured to the buck with two #8 x 1-3/4" screws.	114 lb

The capacities presented in Table 2 will be used to prove acceptable alternate anchors and substrates for the windows.

Alternate Anchorages

Calculations on page 15 determine the design capacity of alternate nail fin installation anchorages for the window. The alternate anchorage capacities are summarized in Table 3.

Table 3 Alternate Anchorage Capacities for Nail Fin Installations

Substrate	Anchor	Capacity	Comments
18 Gauge Steel Stud	#10-16 TEKS Screw	104 lb	1. 33 KSI yield strength stud. 2. Full penetration +3 threads. 3. Limited by pull-out capacity.

Calculations on page 16 through page 19 determine the design capacity of alternate through-frame installation anchorages for the window. The alternate anchorage capacities are summarized in Table 4.

Table 4 Alternate Anchorage Capacities for Through-Frame Installation

Substrate	Anchor	Capacity	Comments
18 Gauge Steel Stud	#10-16 TEKS Screw	152 lb	1. 33 KSI yield strength stud. 2. Full penetration +3 threads. 3. Limited by bending of anchor 4. Use two anchors at sill anchor plate placed min 3" on center.
Concrete	3/16" Tapcon	186 lb	1. Minimum $f'_c = 3,000$ psi 2. 1-1/2" Minimum Embedment 3. 2" Min. Edge Distance 4. Limited by shear capacity 5. Maximum 1x buck strip 6. Use two anchors at sill anchor plate placed min 3" on center.
CMU	3/16" Tapcon	135 lb	1. Minimum ASTM C90 CMU 2. 1-1/2" Minimum Embedment 3. 2" Min. Edge Distance 4. Limited by shear capacity. 5. Maximum 1x buck strip 6. Use two anchors at sill anchor plate placed min 3" on center.

Note: Maximum available length of 3/16" Tapcon anchor is 3-1/4". Use 1/4" x 4" Tapcon anchors for through-frame installations with 1x buck strip.

Anchorage Requirements

It must be determined the anchorages are not overloaded for the approved window size and design pressures. Calculations presented on page 20 show the as-tested spacing is adequate for the minimum anchor capacity reported in this report when the windows are subjected to the maximum design pressures of the products at their approved maximum sizes. Thus, all alternate anchorages proposed by this report may be used for the windows at the as-tested spacing.

Attachments

Appendix A – Revision Log (1 page)

As-Tested Installation – Nail Fin to Wood

#8 x 1-5/8" Wood Screw

PVC Nailing Fin

Spruce-Pine-Fir Wood Substrate Minimum (G=0.42)

Allowable Tension of #8 x 1-5/8" Wood Screw

$$W = 1.6(1.625"-0.062")(82 \text{ lb/in}) \quad (\text{NDS, Table 12.2B})$$
$$W = 205 \text{ lb}$$

Pull-Over of #8 x 1-5/8" Wood Screw

Validated by Testing

Must maintain anchor spacing and anchor head size

As-tested spacing: 8" on center
As-tested anchor head size: 0.334"

Anchor Placement: 3" from corner; 8" on center
Anchor Quantities: 8 each jamb; 9 head; 9 sill; 34 total
Load to Anchors: (74")(63")(50 psf/144) = 1,619 lb
Individual Anchor Load: (1,619 lb)/(34 anchors) = 48 lb (< withdrawal capacity)

Design Capacity of Connection is 48 lb

As-Tested – Through-Frame to Wood

#8 x 2" Wood Screw

PVC Frame; 0.140" thickness at fastener location

1/4" Maximum Shim Space

Spruce-Pine-Fir Wood Substrate Minimum (G=0.42)

Allowable Shear of #8 x 2" Wood Screw

$Z' = 114 \text{ lb}$ (Limited by Mode III_s, See Following 2 Pages)

Bending of #8 x 2" Wood Screw

$L = 1/4"$ (maximum shim space)

$S = \pi d^3/32 = \pi(0.131)^3/32 = 0.000221 \text{ in}^3$

$F_b = (1.3)(0.6F_y) = (1.3)(0.6)(90,000 \text{ psi}) = 70,200 \text{ psi}$ (1.3 for weak axis bending)

$F_b = M/S = (VL/2)/S$ (L/2 for guided bending)

$V = 2SF_b/L = (2)(0.000221 \text{ in})(70,200 \text{ psi})/0.25" = 124 \text{ lb.}$

Capacity of Connection is 114 lb

As-Tested – Through-Frame to Wood (Continued)

Lateral Design Strength of Wood Connections

Data

Fastener			
Fastener	=	#8 Wood Screw	
Shank Dia	=	0.164	in.
Root Dia.	=	0.131	in.
F _{yb}	=	90,000	psi
Fastener length	=	2.000	in.
Main Member			
Material	=	SPF	
G	=	0.42	
θ	=	90	<= (Angle of load to grain 0° ≤ θ ≤ 90°)
F _e	=	3,350	psi
Thickness	=	1.500	in.
Side Member			
Material	=	Vinyl (PVC)	
G	=	N/A	
θ	=	90	<= (Angle of load to grain 0° ≤ θ ≤ 90°)
F _{es}	=	13,750	psi
Thickness	=	0.140	in.

Calculations

Lateral Bearing Factors

D	=	0.131	in.
ℓ _m	=	1.500	in.
K _θ	=	1.25	
K _D	=	2.20	
R _e	=	0.244	
R _t	=	10.71	
k ₁	=	1.0129	
k ₂	=	0.6403	
k ₃	=	5.74	

As-Tested – Through-Frame to Wood (Continued)

Yield Mode	R _d
I _m , I _s	2.20
II	2.20
III _m , III _s , IV	2.20

Lateral Design Values, Z

Mode I _m	=	299	lbf
Mode I _s	=	115	lbf
Mode II	=	116	lbf
Mode III _m	=	129	lbf
Mode III _s	=	71	lbf
Mode IV	=	99	lbf
C _D	=	1.6	

<===== Minimum Value

Wet Service Factor

Fabrication/In-Service	Dry/Dry
C _M	= 1.0
In service temperature	T ≤ 100°F
C _t	= 1.0
C _g	= 1.0
C _Δ	= 1.0
Is fastener installed in end grain?	No
C _{eg}	= 1.00
Is fastener part of a diaphragm?	No
C _{di}	= 1.0
Is fastener toe-nailed?	No
C _{tn}	= 1.00
Z'	= 114 lbf

As-Tested – Anchor Plate to Window Frame

#10 x 1/2" Screw

PVC Frame; 0.070" thickness at fastener location

Steel anchor plate; 0.063" thickness Grade 33 ($F_y = 33,000$ psi, $F_u = 45,000$ psi)

Allowable Shear of #10 x 1/2" Screw

$$V_a = 216 \text{ lb} \quad (\text{AAMA TIR A9-14, Grade 2 Screw})$$

Bearing of #10 x 1/2" Screw on PVC Frame

$$\begin{aligned} V_a &= DtF_p \\ V_a &= (0.190")(0.070")(10,000 \text{ psi}) \\ V_a &= 133 \text{ lb} \end{aligned}$$

Bearing of #10 x 1/2" Screw on Steel Anchor Plate

$$\begin{aligned} V_a &= 2.7DtF_{tu}/\Omega \\ V_a &= 2.7(0.190")(0.063")(45,000 \text{ psi})/3.0 \\ V_a &= 485 \text{ lb.} \end{aligned}$$

Capacity of Connection is 133 lb x 2 screws = 266 lb

For alternate Aluminum Anchor Plate

0.075" thickness (6063-T5, $F_y = 16,000$ psi, $F_u = 22,000$ psi)

Bearing of #10 x 1/2" Screw on Aluminum Anchor Plate

$$\begin{aligned} V_a &= 2DtF_u/n_u \\ V_a &= 2(0.190")(0.075")(22,000 \text{ psi})/3.0 \\ V_a &= 209 \text{ lb.} \end{aligned}$$

Capacity of Connection is 133 lb x 2 screws = 266 lb

Aluminum Anchor Plate will Govern Connection to Substrate

As-Tested – Anchor Plate to Wood

#8 x 1-3/4" Wood Screw

0.075" thickness Aluminum Anchor Plate (6063-T5, $F_y = 16,000$ psi, $F_u = 22,000$ psi)

Spruce-Pine-Fir Wood Substrate Minimum ($G=0.42$)

Allowable Shear of #8 x 1-3/4" Wood Screw

$Z' = 119$ lb (Limited by Mode III_s, See Following 2 Pages)

Capacity of Connection is 119 lb x 2 = 238 lb

As-Tested – Anchor Plate to Wood (Continued)

Lateral Design Strength of Wood Connections

Data

Fastener			
Fastener	=	#8 Wood Screw	
Shank Dia	=	0.164	in.
Root Dia.	=	0.131	in.
F _{yb}	=	90,000	psi
Fastener length	=	1.750	in.
Main Member			
Material	=	SPF	
G	=	0.42	
θ	=	90	<= (Angle of load to grain 0° ≤ θ ≤ 90°)
F _e	=	3,350	psi
Thickness	=	1.500	in.
Side Member			
Material	=	6063 T5 Aluminum	
G	=	N/A	
θ	=	90	<= (Angle of load to grain 0° ≤ θ ≤ 90°)
F _{es}	=	27,500	psi
Thickness	=	0.075	in.

Calculations

Lateral Bearing Factors

D	=	0.131	in.
ℓ _m	=	1.347	in.
K _θ	=	1.25	
K _D	=	2.20	
R _e	=	0.122	
R _t	=	17.96	
k ₁	=	0.8762	
k ₂	=	0.5666	
k ₃	=	10.59	

As-Tested – Anchor Plate to Wood (Continued)

Yield Mode	R_d
I_m, I_s	2.20
II	2.20
III_m, III_s, IV	2.20

Lateral Design Values, Z

Mode I_m	=	269	lbf
Mode I_s	=	123	lbf
Mode II	=	108	lbf
Mode III_m	=	122	lbf
Mode III_s	=	75	lbf
Mode IV	=	104	lbf
C_D	=	1.6	

<===== Minimum Value

Wet Service Factor

Fabrication/In-Service	Dry/Dry
C_M	= 1.0
In service temperature	$T \leq 100^\circ\text{F}$
C_t	= 1.0
C_g	= 1.0
C_Δ	= 1.0
Is fastener installed in end grain?	No
C_{eg}	= 1.00
Is fastener part of a diaphragm?	No
C_{di}	= 1.0
Is fastener toe-nailed?	No
C_{tn}	= 1.00
Z'	= 119 lbf

Alternate Installation – Nail Fin to Steel Stud

#10-16 TEKS Screw

PVC Nailing Fin

Minimum 18 gauge 33 KSI Steel Stud

Allowable Tension of #10-16 TEKS Screw

$$P_{ss}/\Omega = 885 \text{ lb} \quad (\text{ESR-1976})$$

Pull-Over of #10-16 TEKS Screw

Anchor head size: 0.365" > 0.334" Maintain as-tested spacing.

Pull-Out of #10-16 TEKS Screw

$$P_{\text{not}} = 0.85t_c d F_{u2} / \Omega$$

$$P_{\text{not}} = 0.85(0.0428")(0.190")(45,000 \text{ psi}) / 3.0$$

$$P_{\text{not}} = 104 \text{ lb}$$

Capacity of Connection is 104 lb

Alternate Installation – Trough-Frame to Steel Stud

#10-16 TEKS Screw

PVC Frame; 0.140" thickness at fastener location

1/4" Maximum Shim Space

Minimum 18 gauge 33 KSI Steel Stud

Allowable Shear of #10-16 TEKS Screw

$$P_{ss}/\Omega = 573 \text{ lb (ESR-1976)}$$

Bearing of #10-16 TEKS Screw on Frame

$$V_a = DtF_p$$

$$V_a = (0.190")(0.140")(10,000 \text{ psi})$$

$$V_a = 266 \text{ lb}$$

Bearing of #10-16 TEKS Screw on Steel Stud

$$V_a = 2.7DtF_{tw}/\Omega$$

$$V_a = 2.7(0.190")(0.0428")(45,000 \text{ psi})/3.0$$

$$V_a = 329 \text{ lb.}$$

Tilting of #10-16 TEKS Screw in Steel Stud

$$V_a = 4.2(t_2^3D)^{1/2}F_{tu2}/n_s$$

$$V_a = 4.2(0.0428''^3 \times 0.190'')^{1/2}(45,000 \text{ psi})/3.0$$

$$V_a = 243 \text{ lb.}$$

Bending of #10-16 TEKS Screw

$$L = 1/4" \text{ (Maximum Shim Space)}$$

$$S = \pi d^3/32 = \pi(0.139)^3/32 = 0.000264 \text{ in}^3$$

$$F_b = (1.3)(0.6F_y) = (1.3)(0.6)(92,000 \text{ psi}) = 71,760 \text{ psi (1.3 for weak axis bending)}$$

$$F_b = M/S = (VL/2)/S \text{ (L/2 for guided bending)}$$

$$V = 2SF_b/L = (2)(0.000264 \text{ in}^3)(71,760 \text{ psi})/0.25" = 152 \text{ lb.}$$

Capacity of Connection is 152 lb.

Alternate Installation – Trough-Frame to Steel Stud (Continued)

Bearing of #10-16 TEKS Screw on Aluminum Plate at Sill

$$V_a = 2DtF_u/n_u$$

$$V_a = 2(0.190")(0.075")(22,000 \text{ psi})/3.0$$

$$V_a = 209 \text{ lb.}$$

Capacity of Connection is 209 lb x 2 Screws = 418 lb

Alternate Installation – Through-Frame to Concrete

3/16" Tapcon Anchor

1-1/2" Minimum Embedment; 2" Minimum Edge Distance

1/4" Maximum Shim Space

PVC Frame, 0.140" thickness at fastener location

Minimum $f'_c = 3,000$ psi Concrete

Allowable Shear of 3/16" Tapcon Anchor

$$P_{ss}/\Omega = 186 \text{ lb} \quad (\text{NOA 21-0201.06})$$

Bearing of 3/16" Tapcon Anchor on Frame

$$\begin{aligned} V_a &= DtF_p \\ V_a &= (0.170")(0.140")(10,000 \text{ psi}) \\ V_a &= 238 \text{ lb} \end{aligned}$$

Bending of 3/16" Tapcon

$$\begin{aligned} L &= 1/4" \text{ (Maximum Shim Space)} \\ S &= \pi d^3/32 = \pi(0.170")^3/32 = 0.000482 \text{ in}^3 \\ F_b &= (1.3)(0.6F_y) = (1.3)(0.6)(137,000 \text{ psi}) = 106,860 \text{ psi} \text{ (1.3 for weak axis bending)} \\ F_b &= M/S = (VL/2)/S \text{ (L/2 for guided bending)} \\ V &= 2SF_b/L = (2)(0.000482 \text{ in}^3)(106,860 \text{ psi})/0.25" = 412 \text{ lb.} \end{aligned}$$

Capacity of Connection is 186 lb

Qualifies 1/4" Tapcon if longer length anchor is required to achieve minimum embedment.

Bearing of 3/16" Tapcon on Aluminum Plate at Sill

$$\begin{aligned} V_a &= 2DtF_u/n_u \\ V_a &= 2(0.170")(0.075")(22,000 \text{ psi})/3.0 \\ V_a &= 187 \text{ lb.} \end{aligned}$$

Capacity of Connection is 186 lb x 2 Anchors = 372 lb

Alternate Installation – Through Frame to-CMU

3/16" Tapcon Anchor

1-1/2" Minimum Embedment, 2" Minimum Edge Distance

1/4" Maximum Shim Space

PVC Frame, 0.140" thickness at fastener location

Minimum ASTM C90 Concrete Masonry Unit

Allowable Shear of 3/16" Tapcon Anchor

$$P_{ss}/\Omega = 135 \text{ lb} \quad (\text{NOA 21-0201.06})$$

Bearing of 3/16" Tapcon Anchor on Frame

$$\begin{aligned} V_a &= DtF_p \\ V_a &= (0.170")(0.140")(10,000 \text{ psi}) \\ V_a &= 238 \text{ lb} \end{aligned}$$

Bending of 3/16" Tapcon

$$\begin{aligned} L &= 1/4" \text{ (Maximum Shim Space)} \\ S &= \pi d^3/32 = \pi(0.170")^3/32 = 0.000482 \text{ in}^3 \\ F_b &= (1.3)(0.6F_y) = (1.3)(0.6)(137,000 \text{ psi}) = 106,860 \text{ psi} \text{ (1.3 for weak axis bending)} \\ F_b &= M/S = (VL/2)/S \text{ (L/2 for guided bending)} \\ V &= 2SF_b/L = (2)(0.000482 \text{ in}^3)(106,860 \text{ psi})/0.25" = 412 \text{ lb.} \end{aligned}$$

Capacity of Connection is 135 lb

Qualifies 1/4" Tapcon if longer length anchor is required to achieve minimum embedment.

Bearing of 3/16" Tapcon on Aluminum Plate at Sill

$$\begin{aligned} V_a &= 2DtF_u/n_u \\ V_a &= 2(0.170")(0.075")(22,000 \text{ psi})/3.0 \\ V_a &= 187 \text{ lb.} \end{aligned}$$

Capacity of Connection is 135 lb x 2 Anchors = 270 lb

Anchorage Requirements

74 x 63 HS – Fin Install

Anchor Placement:	3" from corner; 8" on center
Anchor Quantities:	8 each jamb; 9 head; 9 sill; 34 total
Load to Anchors:	$(74")(63")(50 \text{ psf}/144) = 1,619 \text{ lb}$
Individual Anchor Load:	$(1,619 \text{ lb})/(34 \text{ anchors}) = 48 \text{ lb}$
Least Anchor Capacity:	104 lb > 48 lb <u>OK</u>

74 x 63 HS – Finless Install

Stile Load Area at Sill Plate:	$[(37"/2)(63") - 37"{}^2/4]/144 = 5.7 \text{ ft}{}^2$
Load at Sill Plate:	$(50 \text{ psf})(5.7 \text{ ft}{}^2) = 285 \text{ lb}$
Least Anchor Capacity:	135 lb. <u>Specify 2 anchors for Sill Plate</u>
Perimeter Anchors:	6 head, 5 each jamb; 16 total
Load to Perimeter Anchors:	$1,619 \text{ lb} - 285 \text{ lb} = 1,334 \text{ lb}$
Individual Anchor Load:	$(1,334 \text{ lb})/(16 \text{ anchors}) = 83 \text{ lb}$
Least Anchor Capacity:	114 lb > 83 lb <u>OK</u>

Appendix A

Revision Log

<u>Identification</u>	<u>Date</u>	<u>Page & Revision</u>
Original Issue	12/11/23	Not Applicable